BIRLA INSTITUTE OF TECHNOLOGY, MESRA, RANCHI (END SEMESTER EXAMINATION)

CLASS: BRANCH	B.TECH : CEE	SEMEST SESSIO	SEMESTER : IV SESSION : SP/2023		
TIME:	SUBJECT: CE303 GEOTECHNICAL ENGINEERING 3 Hours		FULL MARKS: 50		
INSTRUC 1. The q 2. Atterr 3. The n 4. Befor 5. Table	CTIONS: question paper contains 5 questions each of 10 marks and total 5 npt all questions. nissing data, if any, may be assumed suitably. re attempting the question paper, be sure that you have got the o s/Data hand book/Graph paper etc. to be supplied to the candida	0 marks. correct question pap ates in the examinati	er. ion h	all.	
Q.1(a) Q.1(b)	Describe the corrections to be applied to hydrometer readings and for applying the corrections? An oven dry soil sample of volume 220 cm ³ has a mass of 400g. Dete and shrinkage limit if the specific gravity of soil particles is assu sample is allowed to swell and get fully saturated on contact with the water content which will fully saturate the sample and cause a	explain the reasons ermine its voids ratio med as 2.76. If the water, what will be	[5] [5]	CO 1 1	BL 2 3
Q.2(a) Q.2(b)	Derive the equation for coefficient of permeability for a stratified is a) parallel to planes b) perpendicular to planes A building column has a footing area of $3m \times 4m$ and transmits a per KN/m ² and its base is embedded 2 m below GL. Assuming a pressur- vertical to 1 horizontal, determine the consolidation settlement at layer. W.T at a depth of 1.5 m below the base of footing. Sand -Y = 20.5 KN/m ³ , thickness-4m. Clay - Y _{sat} = 18KN/m ³ , e ₀ =0.90, C _c =0.22	layer when the flow ressure of 200 re distribution of 2 t the middle of clay = 17.5KN/m ³ & Y _{sat} = 8 thickness- 3m	[5] [5]	2 2	2 3
Q.3(a) Q.3(b)	Explain the limitations of direct shear test over triaxial test. A cylindrical soil specimen having cohesion $80kN/m^2$ and angle of is subjected to a cell pressure of $100kN/m^2$ in a triaxial testing ma maximum deviator stress at which the sample will fail and the angle plane with the axis of the specimen.	internal friction 21 ⁰ achine. Compute the a made by the failure	[5] [5]	3 3	1 3
Q.4(a) Q.4(b)	Explain how the drawback of SPT N value taken care in Cone Pener A strip footing, 2m wide, located at a depth of 1.5 m in a 20kN/m ³ .Water table is quite close to the ground surface. Determ capacity, using a factor of safety 2.85 c=15kN/m ² and Ø=28 ⁰ N _c = 2 16.72	tration test. clay soil with Y _{sat} = ine the safe bearing 5.80, N _q = 14.72, N _Y =	[5] [5]	4 4	2 3
Q.5(a) Q.5(b)	Summarize the method of proportioning of footing size with respect Design a friction pile group to carry a load of 300t including the we Average unconfined compressive strength of the clay is 0.7 kg/cm and diameter of pile 60 cm. A factor of safety 3 is required against	ct to column loads gight of the pile cap. ² .Length of pile 14m shear failure. α =0.4	[5] [5]	5 5	2 3

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